

Numerical evaluation of fragility curves for earthquake-liquefaction-induced settlements of an embankment

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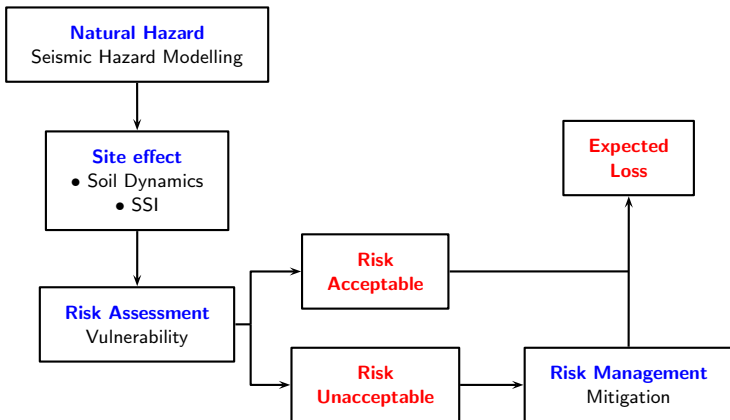
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Dams and Earthquakes ITCOLD and EWG, 2017, Rome

4 février 2017

Context

Earthquake loss estimation



Global approach

- Need for understanding mechanisms controlling induced damage in earthquake loss estimation (e.g. soil foundation, structures, dams, ...) ;
 - Improve and validate traditional approaches and evaluation methods ;
 - Use of numerical methods in order to facilitate the comprehension of the global problem via parametric analyses ;
-
- *Various uncertainties on the material properties, loading parameters and scenarios will be considered ;*
 - *Probabilistic analyses as a complement of conventional deterministic analyses will be used.*

Outline

- 1 Liquefaction simulation
- 2 Liquefaction seismic risk analysis

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GEFDyn - ECP's numerical tool

The ECP's elastoplastic multi-mechanism model [Aubry et al., 1982, Hujeux, 1985]

- The model is written in terms of effective stress,
- Coulomb type failure criterion,
- Critical state concept,
- Deviatoric primary yield surface of the k plane : $f_k(\sigma, \varepsilon_v^p, r_k) = q_k - \sin \phi'_{pp} \cdot p'_k \cdot F_k \cdot r_k$
 $F_k = 1 - b \ln \left(\frac{p'_k}{p_c} \right)$ and $p_c = p_{co} \exp(\beta \varepsilon_v^p)$
- Progressive friction mobilization with shear : $r_k = r_k^{el} + \frac{\int_0^t \overline{\varepsilon^p} dt}{a + \int_0^t \overline{\varepsilon^p} dt}$
 $a = a_1 + (a_2 - a_1) \alpha_k(r_k)$
- Roscoe's dilatancy law
- Isotropic yield surface : $f_{iso} = |p'| - d p_c r_{iso}$

Studied case

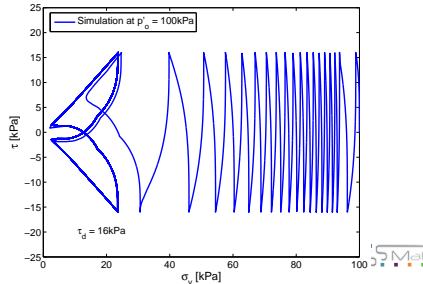
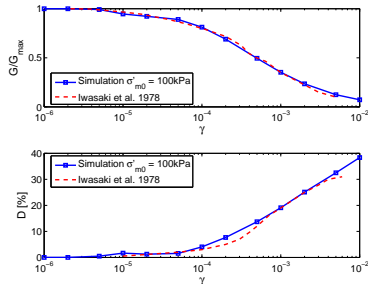
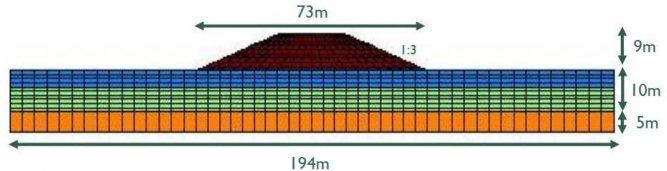
Height of the dam = 9m

Loose sand = 4m

Dense sand = 6m

Water table = 1m
(dam is kept dry)

Bedrock = 5m ($V_s = 1000$ m/s)

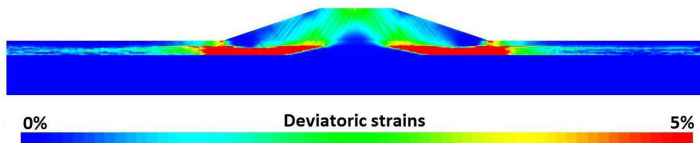
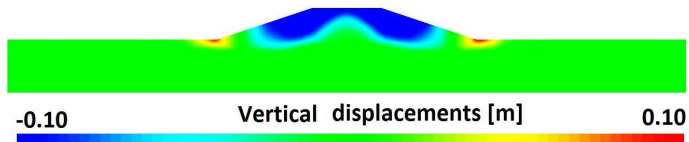


Excess pore water pressure ratio generation

Crest settlement and $r_u = \Delta p_w / \sigma'_{v,0}$ time evolution

EQ3 : $a_{max}=0.42g$, $t_{595}=4.3s$, $N_{cycles}=5$

Studied case



Liquefaction risk assessment

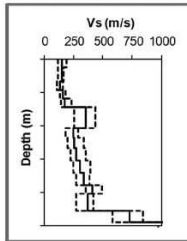
Questions that need to be answered :

- Is the soil susceptible to liquefaction ?
- If yes, Is the apparition of soil liquefaction possible ?
 - ⇒ For what hazard level ?
- If yes, Can damage occur ?
 - ⇒ What kind of damages ?
- Are those damages acceptable ?
 - ⇒ If no, How to minimize the damage ?

Sources of uncertainty

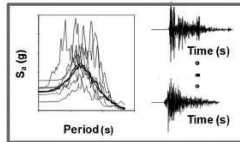
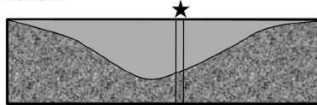
Sources of uncertainty in a seismic analysis

Shear Wave Velocity Characterization



Selection of Method of Analysis

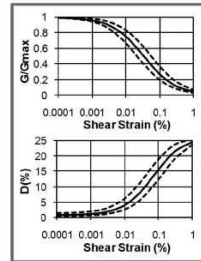
Equivalent-linear, Nonlinear
1D, 2D, 3D



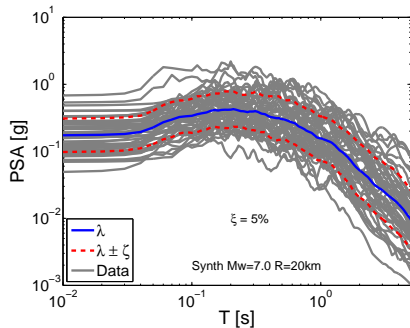
Specification of Input Motions

(Rathje et al 2010)

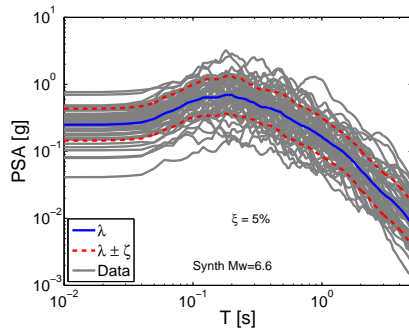
Nonlinear Property Characterization



Seismic Hazard



Synthetic signals M=7.0 - R=20km

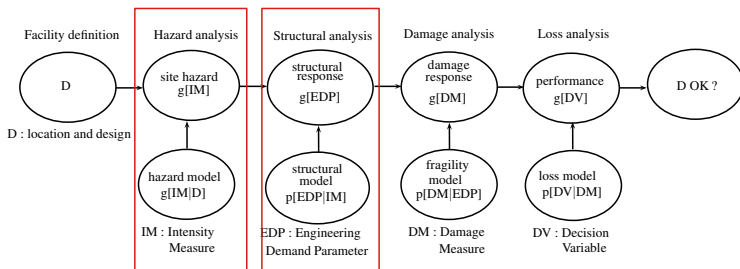


Synthetic signals M=6.6 - R=16km

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PEER's Performance-Based Earthquake Engineering Methodology

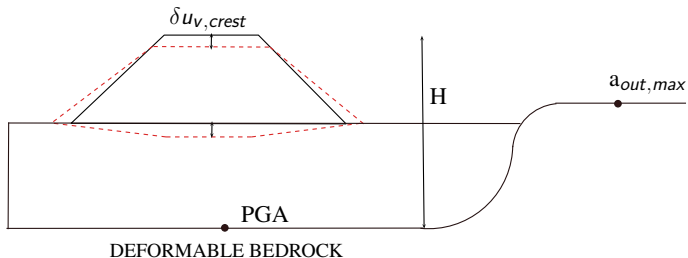


Liquefaction vulnerability analysis : identify the EDP of interest and IM for the seismic hazard

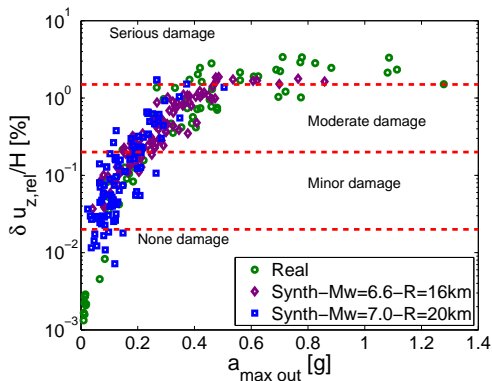
IM : maximum amplitude $a_{bed,max}$, Arias intensity (I_{Arias}), equivalent predominant period $T_{V/A}$, duration of mainshock t_{595} , number of cycles N_{cycles}

EDP of interest : settlements, excess pore water pressure ratio r_u , horizontal displacements, ...

Damage level evaluation

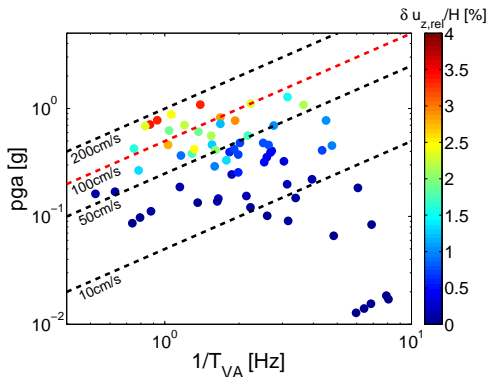


Vulnerability



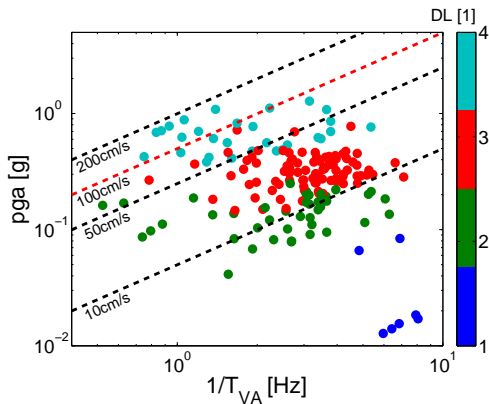
EDP : Crest settlement - IM : $a_{\max out}$

Vulnerability



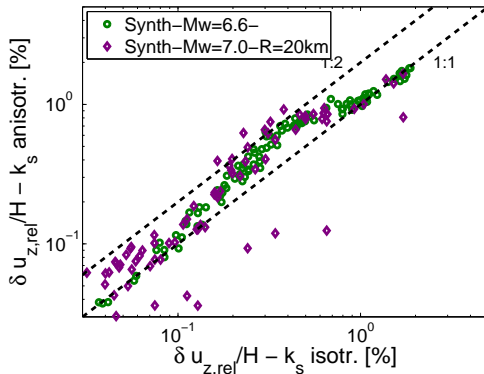
EDP : Crest settlement - IM's : $a_{max out}$, pgv and $T_{V,A}$

Vulnerability



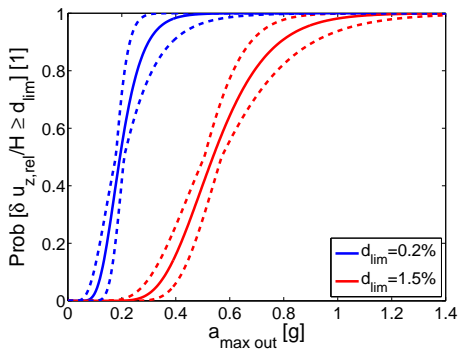
Damage level - IM's : $a_{max out}$, pgv and $T_{V,A}$

Vulnerability

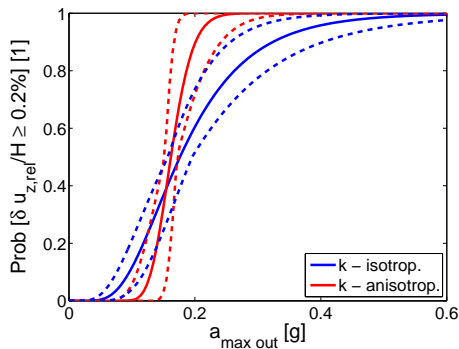


EDP : Crest settlement - Effect of k_s anisotropy - $k_h/k_v = 100$

Vulnerability : Crest settlement



Effect of damage level

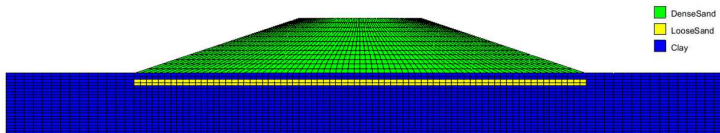


Effect of k_s anisotropy

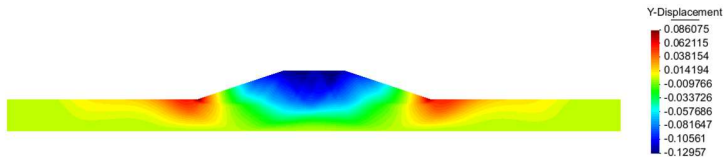
Further works

Liquefaction of soil inside levees :

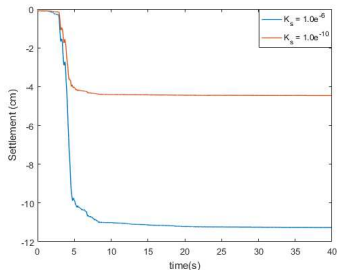
- River levees resting on nonliquefiable foundation
- Soil in the saturated zone liquefies and causes deformation to the levee
 - ⇒ effects of the thickness of the saturated zone
 - ⇒ effects of drainage at the boundary of the saturated zone



effects of drainage boundary conditions



$$k_{s\text{ clay}} / k_{s\text{ sand}} = 1 \times 10^{-2} \text{ and } 1 \times 10^{-6}$$



Thank you for your attention
Merci de votre attention



Aubry, D., Hujeux, J.-C., Lassoudière, F., and Meimon, Y. (1982).

A double memory model with multiple mechanisms for cyclic soil behaviour.

In *International symposium on numerical models in geomechanics*, pages 3–13. Balkema.



Hujeux, J.-C. (1985).

Une loi de comportement pour le chargement cyclique des sols.

In *Génie Parasismique*, pages 278–302. V. Davidovici, Presses ENPC, France.