

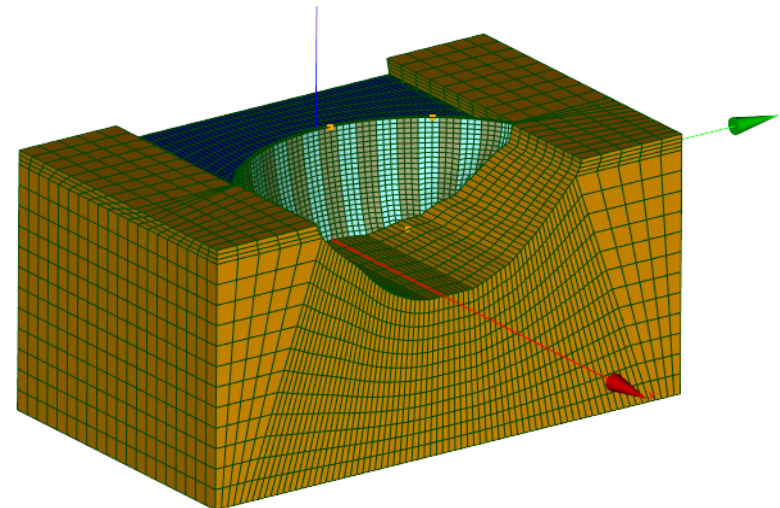
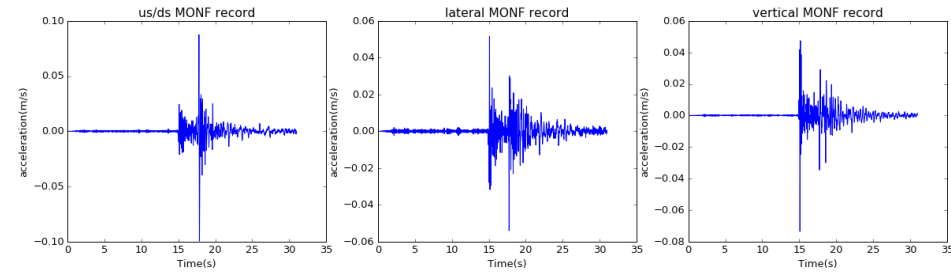
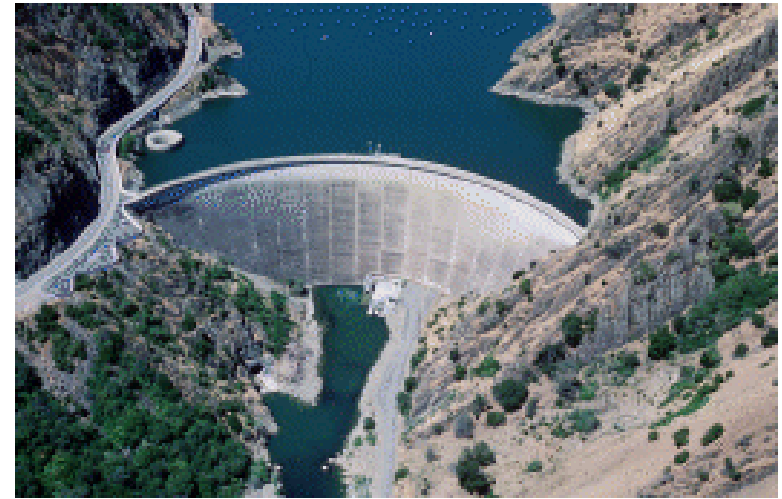


Hydro Engineering Center

BACK ANALYSES OF MONTICELLO ARCH DAM:

interest of earthquake
records on dams to improve
the reliability of safety
assessments on concrete
dams

Emmanuel ROBBE – 06/02/2017



SUMMARY

1. INTRODUCTION AND CONTEXT

2. MONTICELLO ARCH DAM WORKSHOP

3. FINITE ELEMENT METHOD

- SOIL-STRUCTURE INTERACTION : VISCOUS-SPRING BOUNDARIES METHOD
- FLUID-STRUCTURE INTERACTION : POTENTIAL FLUID APPROACH

4. BACK ANALYSES OF MONTICELLO ARCH DAM

- BLIND PREDICTION AND COMPARISON
- ADDITIONAL ANALYSES

5. BACK ANALYSES OF OTHER CONCRETE DAMS

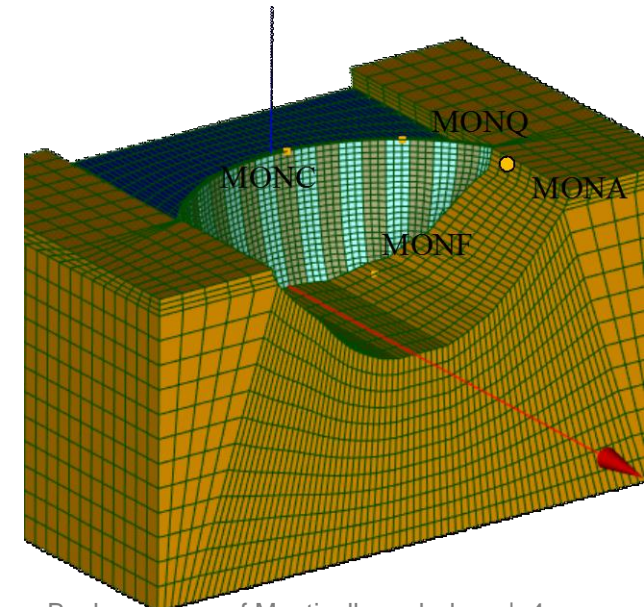
6. CONCLUSION

INTRODUCTION AND CONTEXT

- **WHY ? Safety assessment of existing dams under seismic load**
- **HOW ? Improving our knowledge about dynamic behavior of concrete dams - improving and assessing the calculation methods : until recently, no 'real' data to evaluate our method !!!**
- **MEANS ?**
 - Participation at international benchmarks (ICOLD 2013&2015, USSD workshop on Monticello Dam 2016)
 - Ambient vibration tests on dams (2 gate-structures dams, 1 arch, 1 multiple arch) in 2015 and 2016
 - Research on the spatial variability of the seismic ground motion
 - Collaboration CFBR – JCOLD
 - Analysis on Acceleration Data of Dams Collected by JCOLD (135 gravity dams with 223 earthquake records, 22 arch dams with 59 records)
 - Comparison between records and FE analyses for Tagokura gravity dam and Kurobe arch dam.

MONTICELLO ARCH DAM WORKSHOP

- Blind prediction workshop organized in 2016 by USSD
- M4.1 earthquake recorded in 2015, 16km away from Monticello arch dam (93m high, California)
- Records provided at the toe of the dam (MONF) + shaking test + mechanical tests on concrete & rock core + drawings of dam
- Finite-Element analyses to compute accelerations at the crest (blind prediction)
- Comparison against measured data



PRESENTATION OF IMPROVED FINITE ELEMENT ANALYSES

SOIL-STRUCTURE AND FLUID-STRUCTURE INTERACTION APPROACHES

■ References :

□ Viscous-spring boundary model

- Influence of seismic input mechanisms and radiation damping on arch dam response (Zhang Chuhan 2009)
- Earthquake Response analysis of a gravity dam considering the radiation damping of infinite foundation (Y.S. Liu 2013)

□ Potential-based fluid method

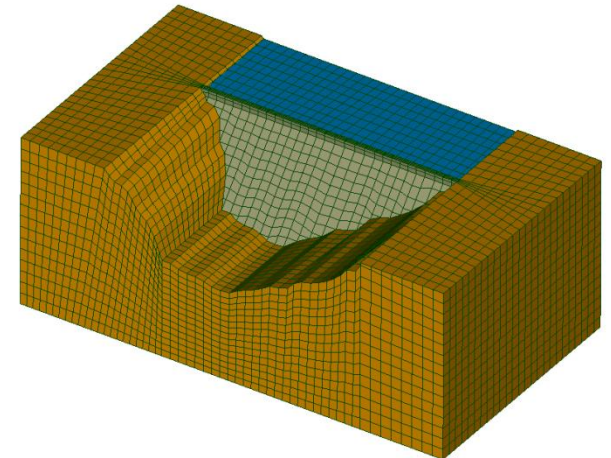
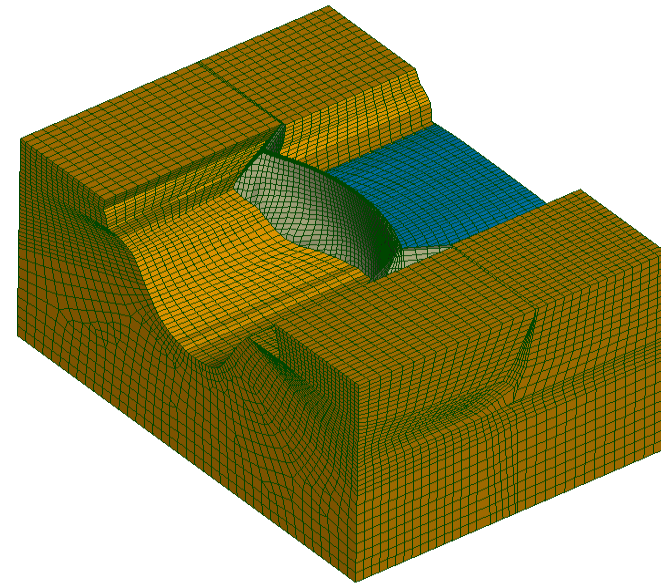
- Assessment of a potential-base fluid finite elements for seismic analysis of dam-reservoir systems (Najib Bouaanani 2008)

■ Test cases

- Comparison of simple to more complex test cases issue from the previous references

■ Software :

- Analyses are carried out with Code_Aster (made by EDF, open-source)



FINITE ELEMENT METHODS

■ Comparison of Time-history analyses with 2 methods

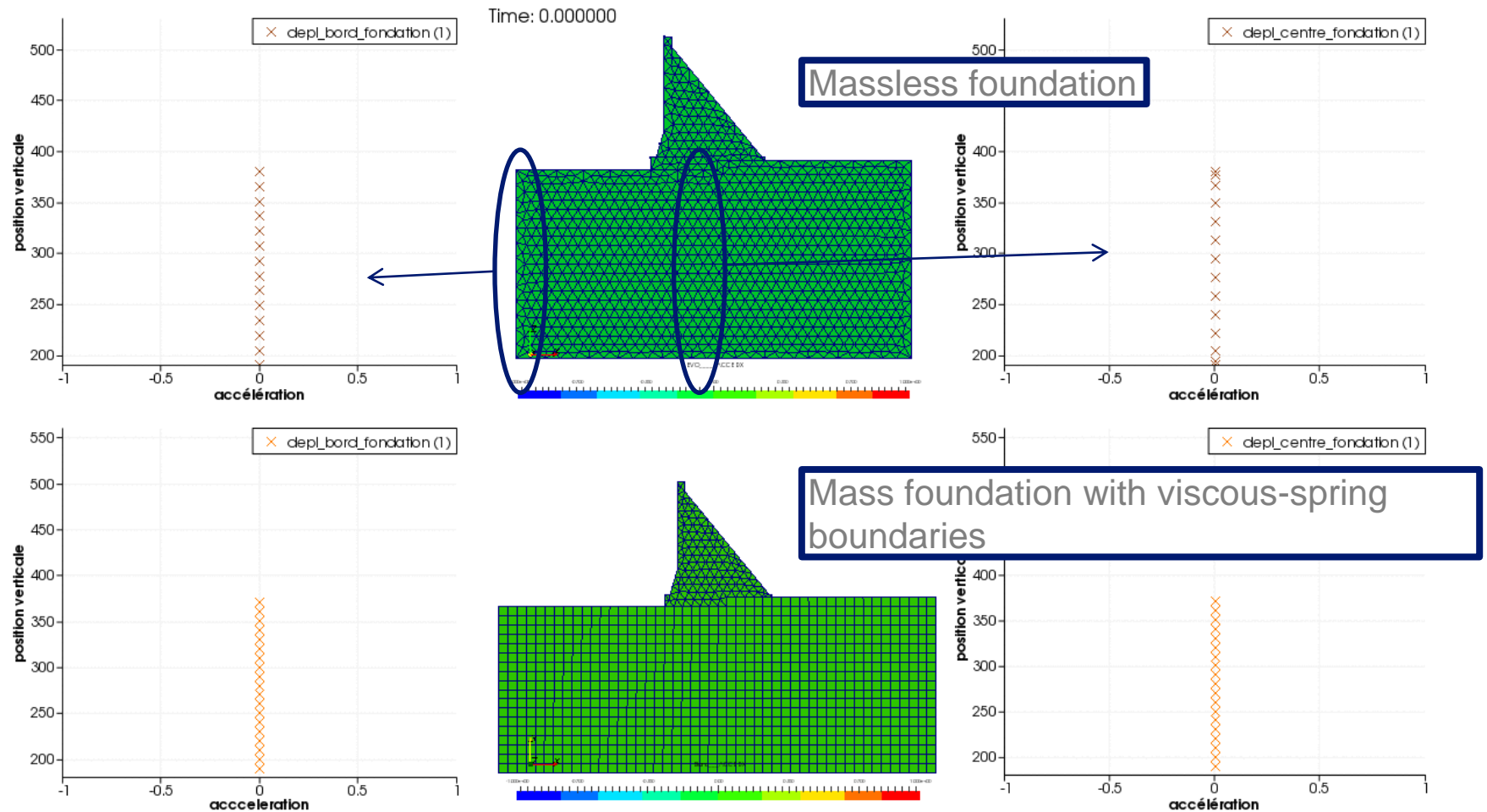
Massless foundation +
Westergaard added masses

- commonly used in engineering practice (ex. CIGB workshop in Lausanne 2015), easy to use (modal analyses..)
- 1 damping source only : concrete material damping (usually 5%)
- the whole foundation is roughly subjected to the same acceleration

mass foundation + viscous-spring-
boundaries + fluid finite element

- less used and more complex :
 - + take into account the propagation of the wave in the foundation and radiative damping
 - + compressibility of the water
- concrete material damping (1-5%), radiation damping in the foundation and in the reservoir
- waves spreads vertically from the bottom of the foundation to the top.

VISUALISATION OF THE EARTHQUAKE INPUT



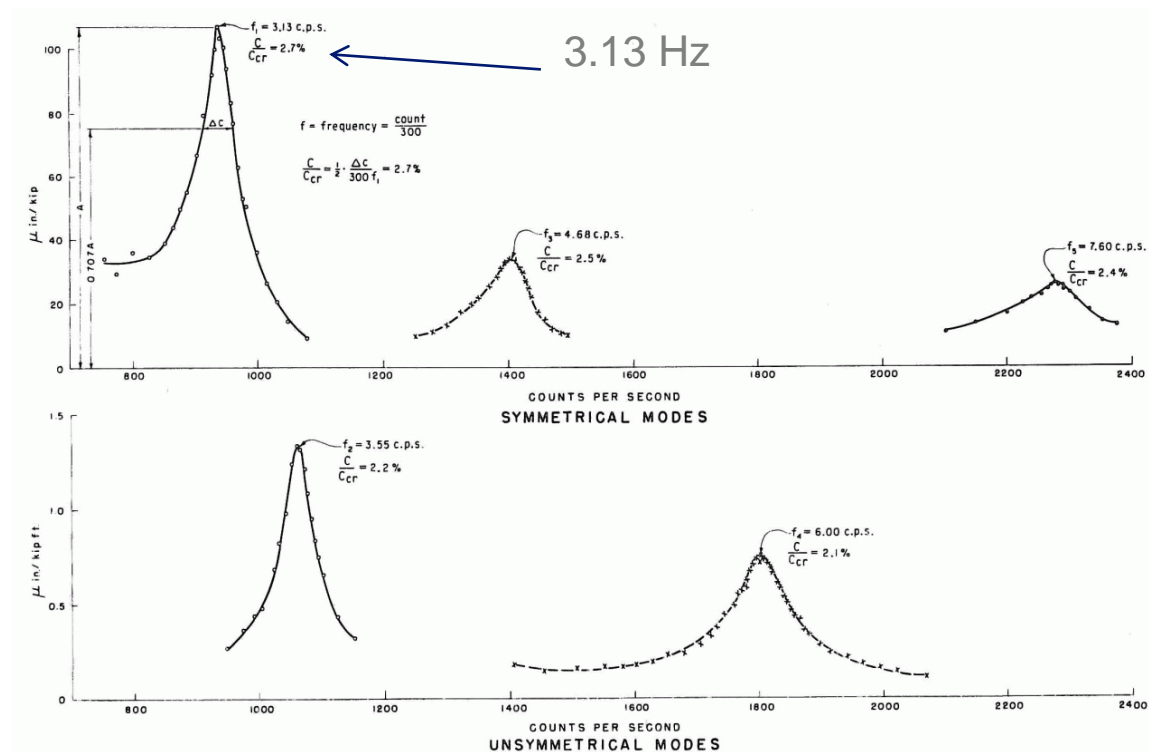
BACK ANALYSIS OF MONTICELLO ARCH DAM

MATERIAL PROPERTIES AND CALIBRATION OF THE MODEL

■ From concrete and rock mechanical test :

- 2450 kg/m³ density
- $E_{\text{concrete}} = 35000 \text{ MPa}$, $E_{\text{rock}} = 30000 \text{ MPa}$

■ Adjusted by shaking test :



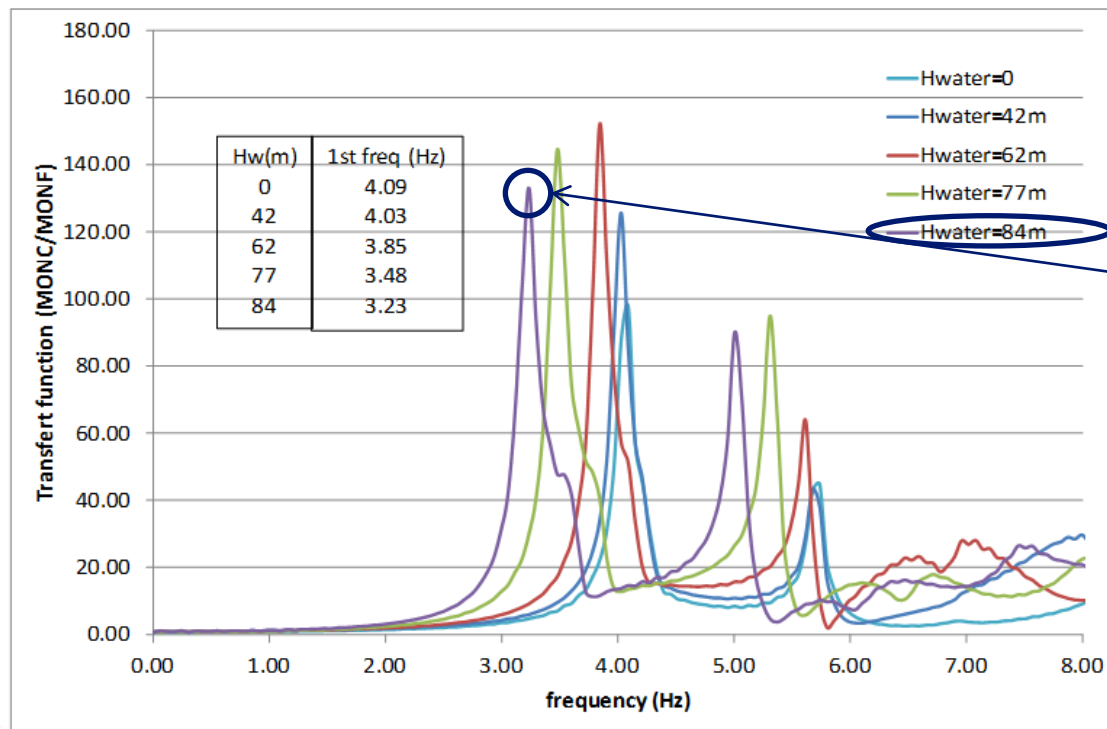
BACK ANALYSIS OF MONTICELLO ARCH DAM

MATERIAL PROPERTIES AND CALIBRATION OF THE MODEL

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■ Adjusted by shaking test :



Transfert function between crest / toe of the dam for different water level :

For the water level of the shaking test :

$F = 3.23 \text{ Hz}$ with $E_c = 35000 \text{ MPa}$
 $F = 3.13 \text{ Hz}$ with $E_c = 30000 \text{ MPa}$

BACK ANALYSIS OF MONTICELLO ARCH DAM

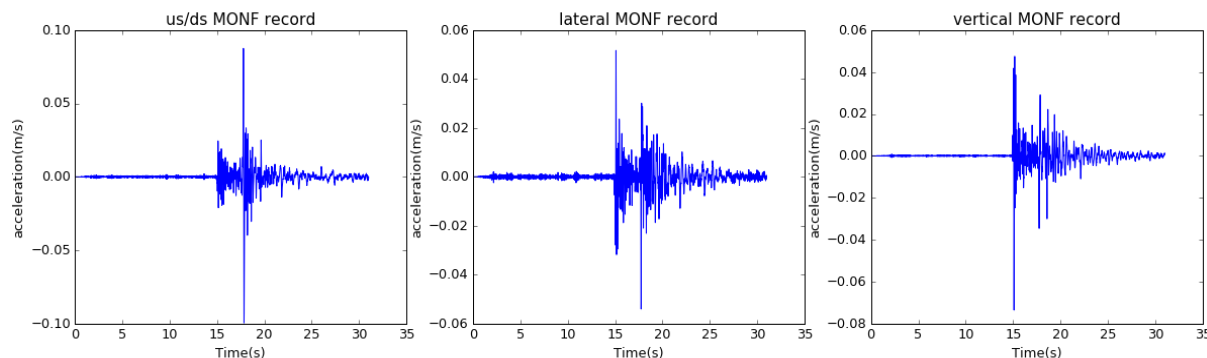
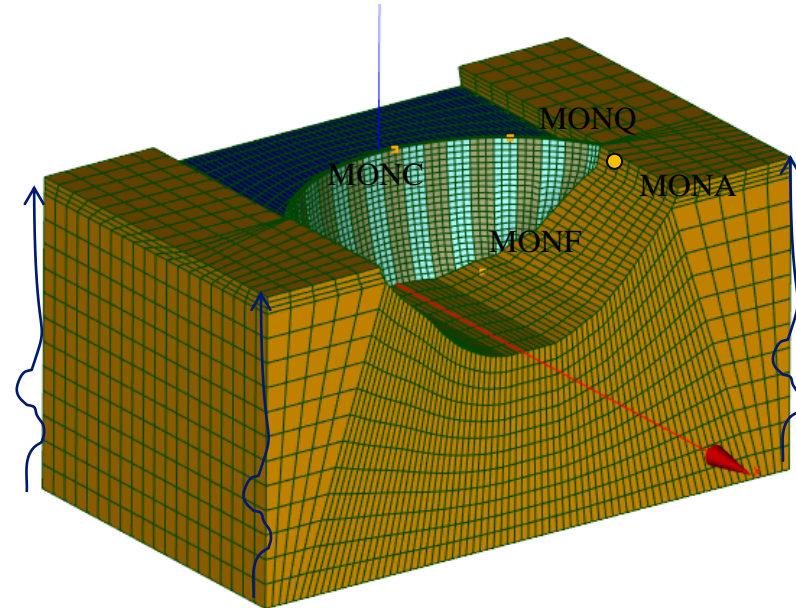
MATERIAL PROPERTIES AND CALIBRATION OF THE MODEL

- **From concrete and rock mechanical test :**
 - 2450 kg/m³ density
 - $E_{\text{concrete}} = 35000 \text{ MPa}$, $E_{\text{rock}} = 30000 \text{ MPa}$
 - **Adjusted by shaking test :**
 - **Concrete damping**
 - Between 1 – 5% (from similar comparison between records and FE analyses) : 1% chosen here due to the very low intensity of the earthquake.
 - **Additional damping due to the viscous-spring boundaries around the foundation**
- **Overall : total damping is generally higher than 5% to fit the records**

BACK ANALYSIS OF MONTICELLO ARCH DAM

SEISMIC INPUT

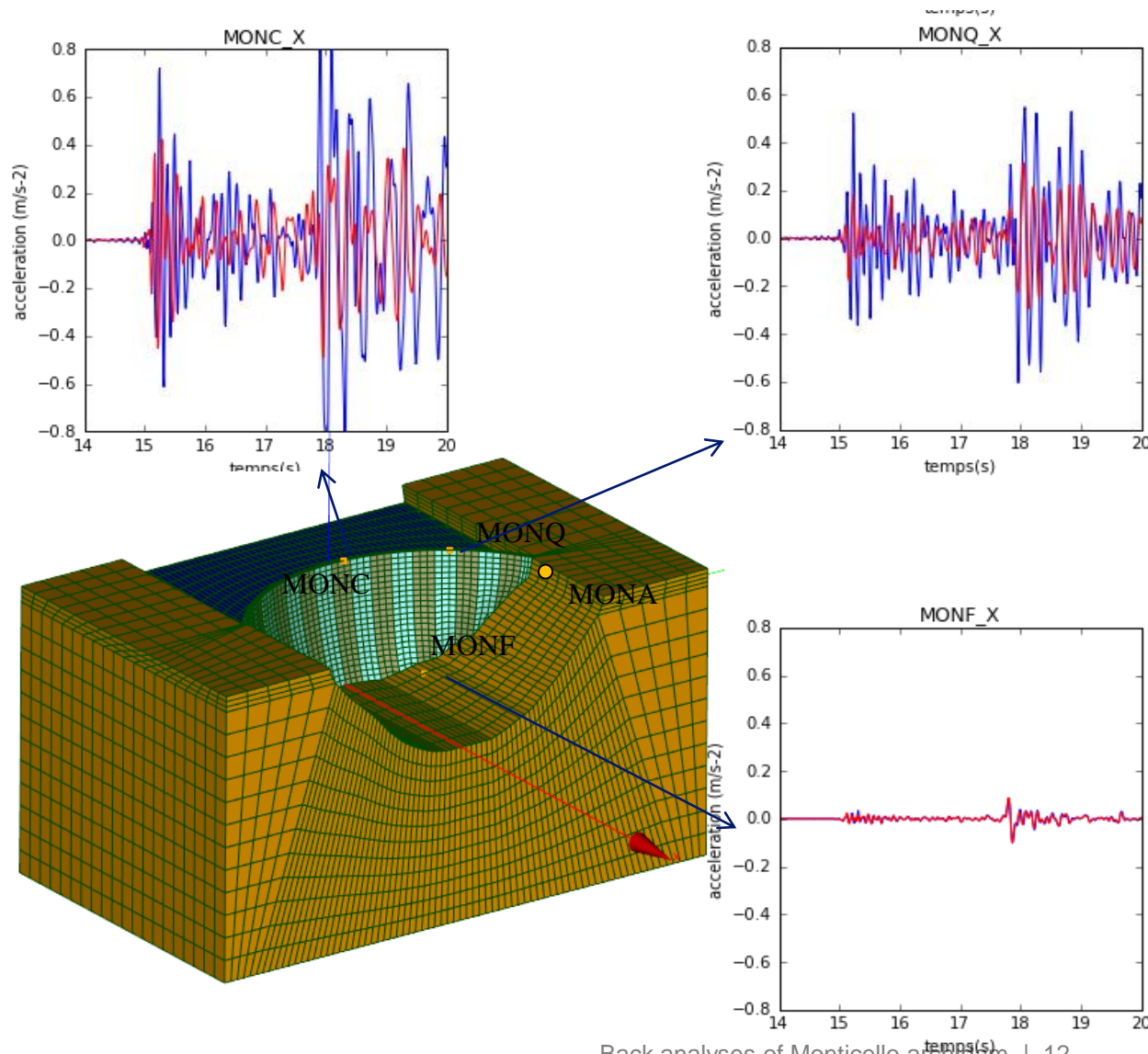
- Acceleration time histories are introduced as compression and shear waves travelling vertically from the bottom to the top of the foundation
- Deconvolution process to define input such as accelerations computed at the toe of the dam fit the accelerations recorded



BACK ANALYSIS OF MONTICELLO ARCH DAM

TIME-HISTORY ACCELERATION COMPARAISON

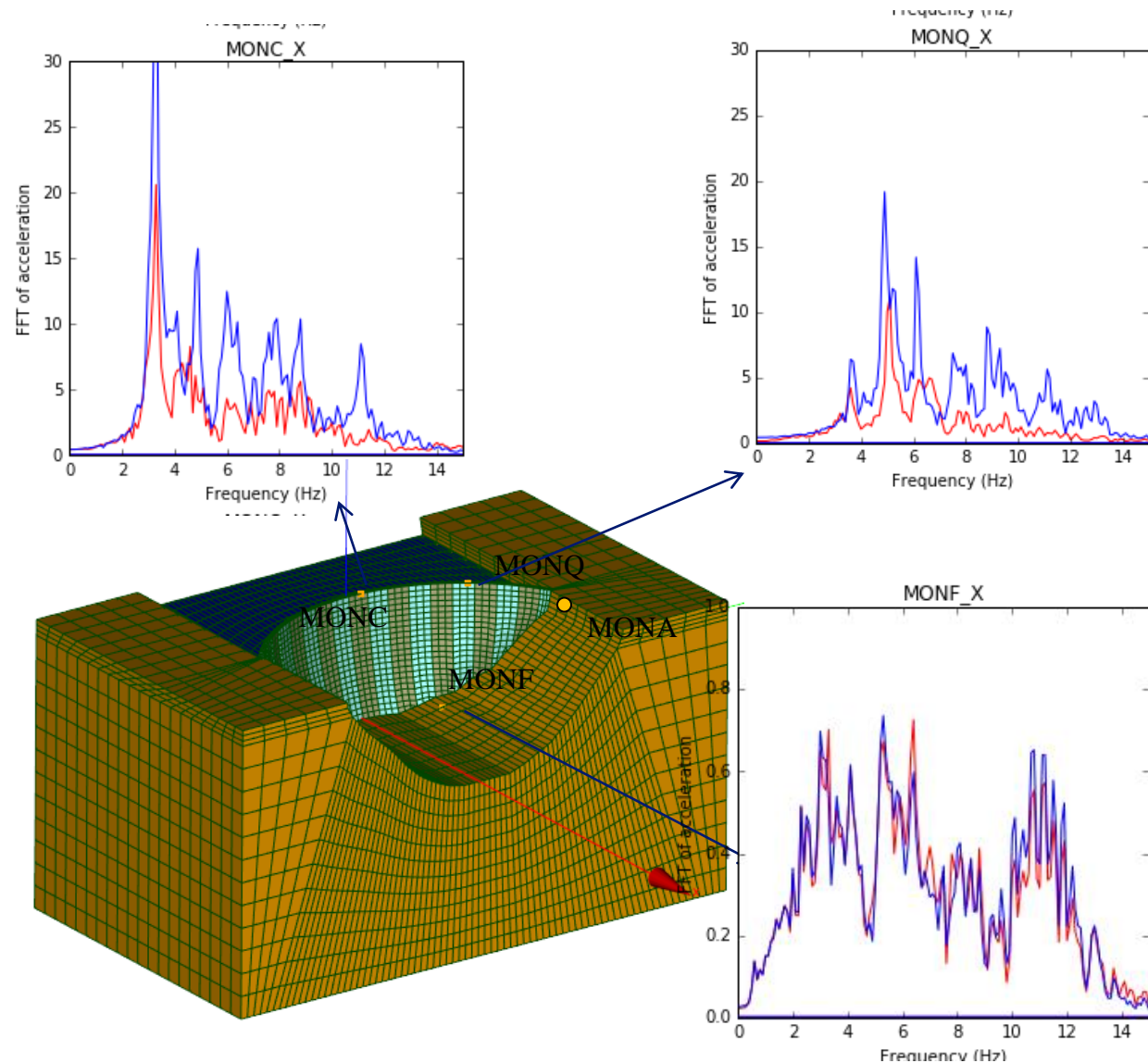
- Blue computed
- Red recorded
- Input correctly introduced
- At the crest : FE model overestimates the response of the dam (x2)



BACK ANALYSIS OF MONTICELLO ARCH DAM

FREQUENCIES COMPARAISON

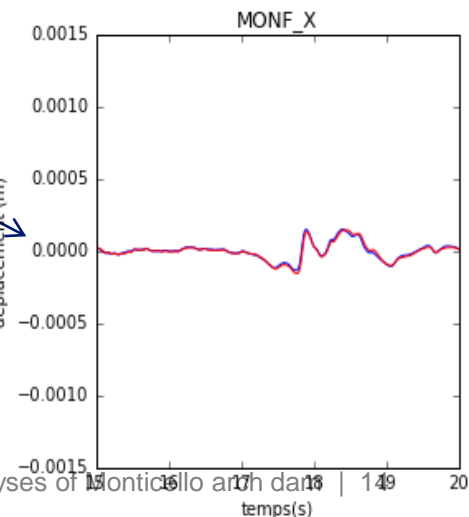
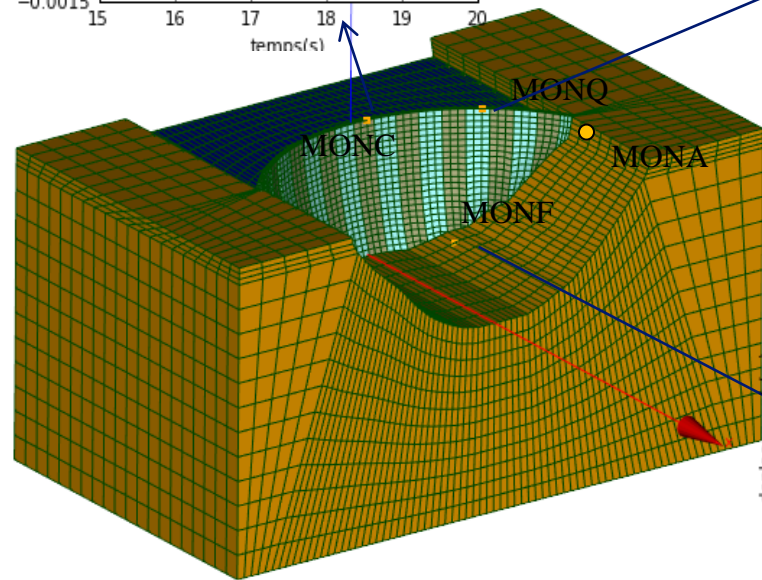
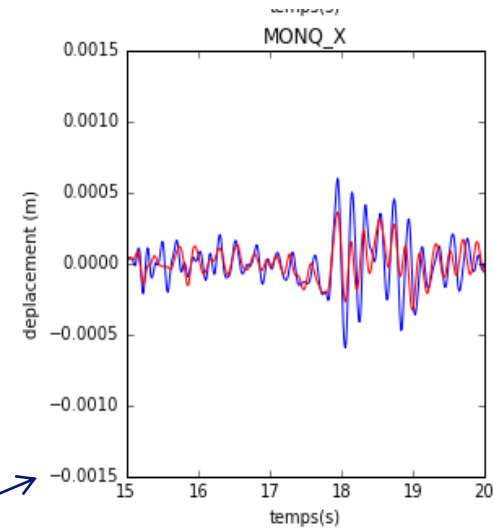
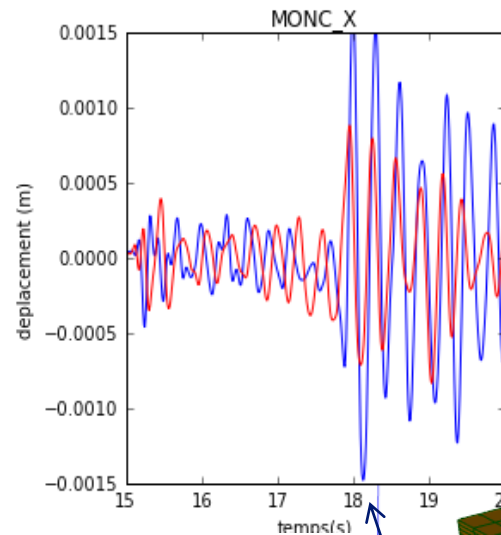
- Blue computed
- Red recorded
- Frequencies of modes are correctly predicted
- Overestimation of the amplitude



BACK ANALYSIS OF MONTICELLO ARCH DAM

DISPLACEMENT COMPARAISON

- Blue computed
- Red recorded
- Overestimation of the displacements at the crest



Back analyses of Monticello arch dam | 149

BACK ANALYSIS OF MONTICELLO ARCH DAM

CONCLUSION OF THE PREDICTION AND ADDITIONAL ANALYSES PROPOSED

■ **Prediction cannot be considered completely satisfying:**

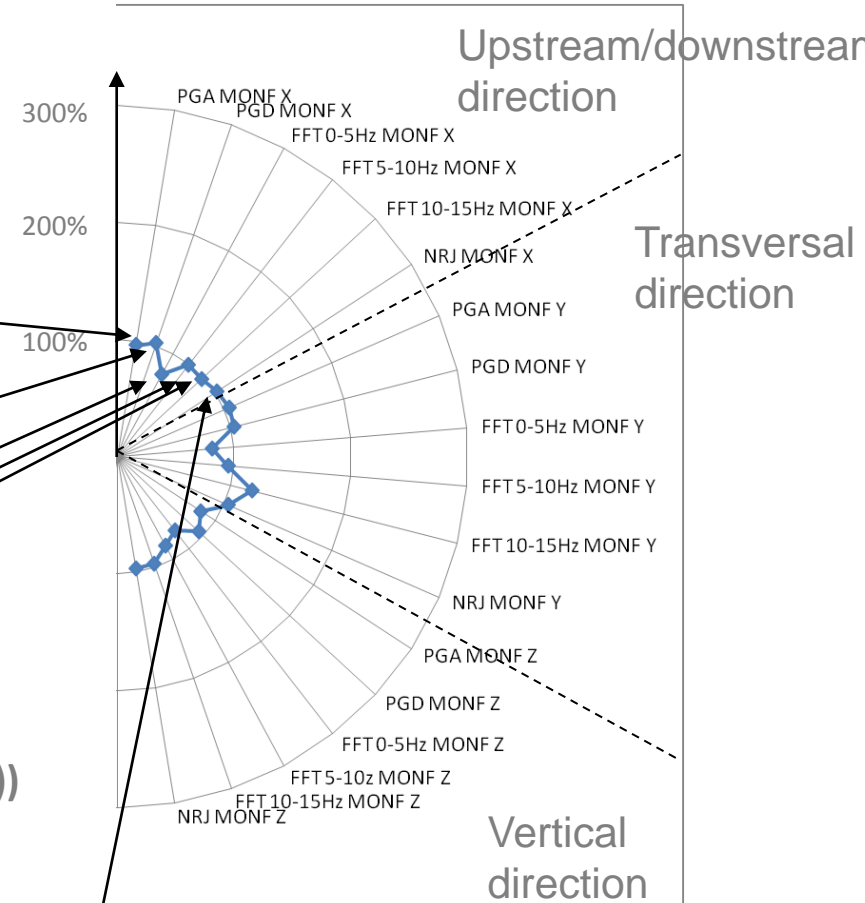
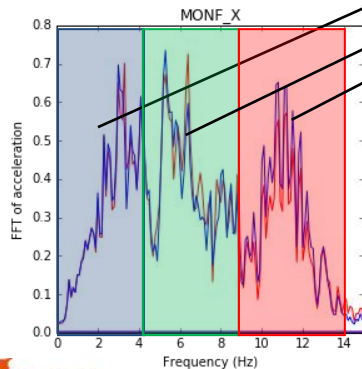
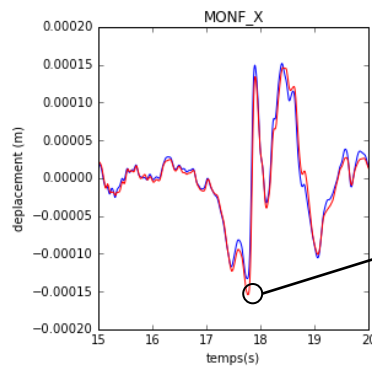
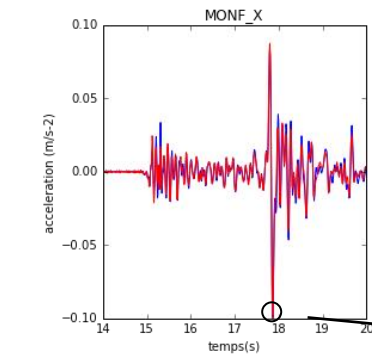
- Input correctly introduced in the FE model
- Accelerations computed at the crest higher than the recorded ones.
- Eigenfrequencies are correctly computed, but amplitudes are not.
- Displacements at the crest, particularly in the upstream/downstream direction, are overestimated.

■ **Additional analyses proposed**

- Correction of the Rock Young Modulus
- Influence of the concrete damping
- Partial absorption of pressure wave at the reservoir bottom
- Change of the earthquake input

ADDITIONNAL ANALYSES FOR MONTICELLO ARCH DAM

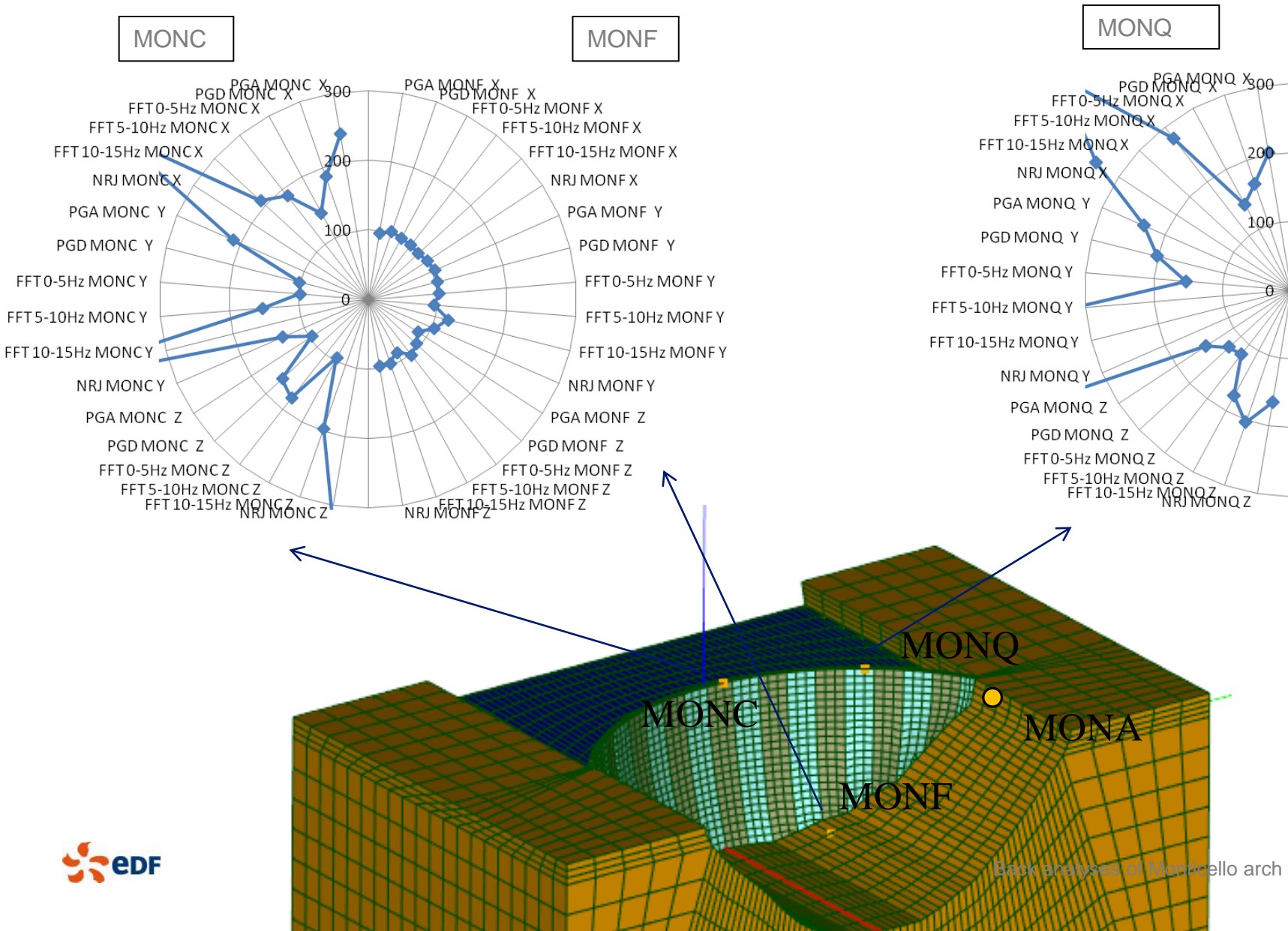
HOW TO COMPARE ?



$$\int_0^t v_{comp}^2 \tau d\tau / \int_0^t v_{rec}^2 \tau d\tau$$

BACK ANALYSIS OF MONTICELLO ARCH DAM

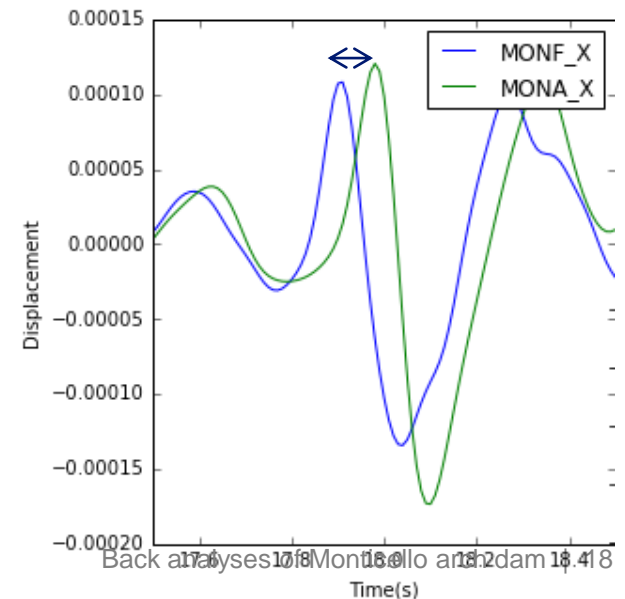
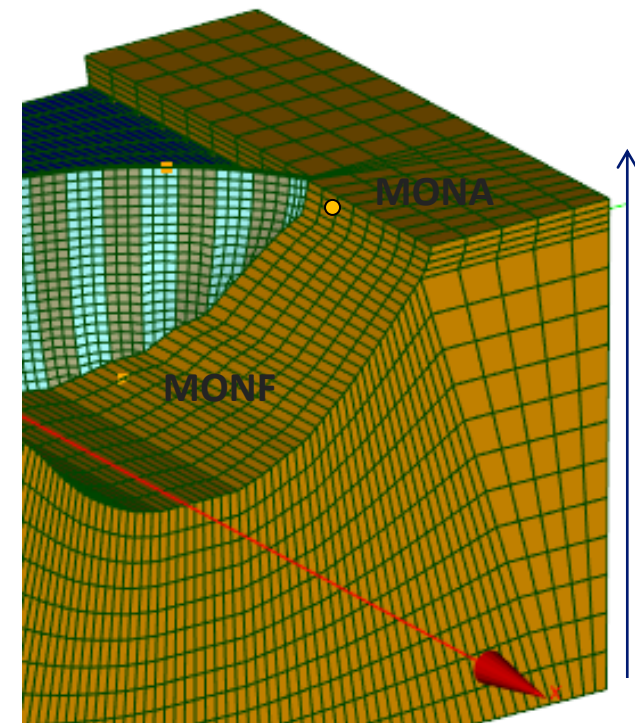
COMPARISON OF THE INITIAL PREDICTION AGAINST MEASURED DATA



BACK ANALYSIS OF MONTICELLO ARCH DAM

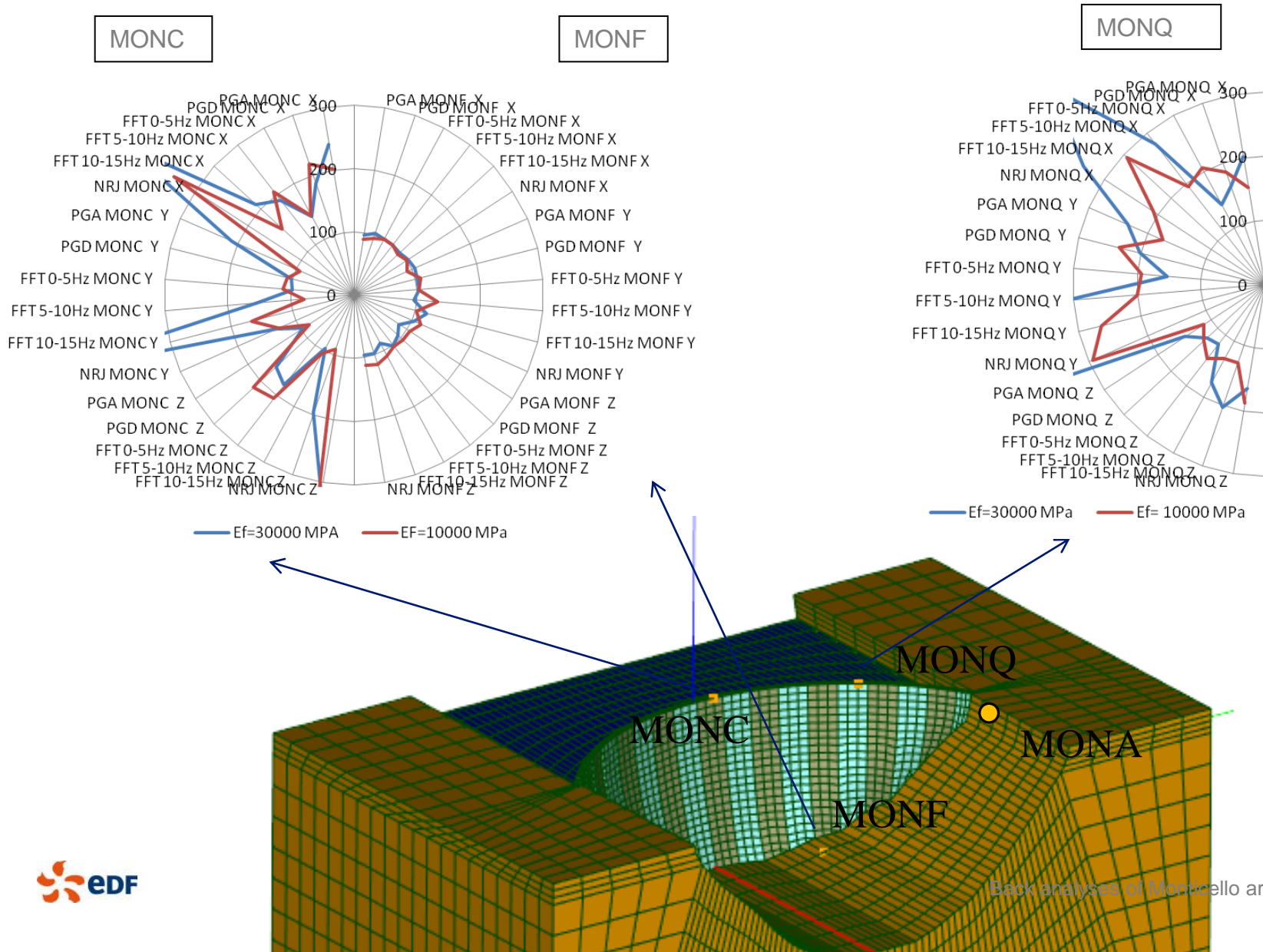
CORRECTION OF THE ROCK YOUNG MODULUS

- With recording devices at different levels of the foundation, it's possible to get some information of the wave velocity in the rock :
 - From displacement : delay for the shear and compression waves to travel vertically from MONF to MONA
 - 0.06s for shear waves
 - 0.04s for compression waves
 - Vertical distance between devices : 84m
- Dynamic Rock Modulus : 10 000 MPa



BACK ANALYSIS OF MONTICELLO ARCH DAM

CORRECTION OF THE ROCK YOUNG MODULUS



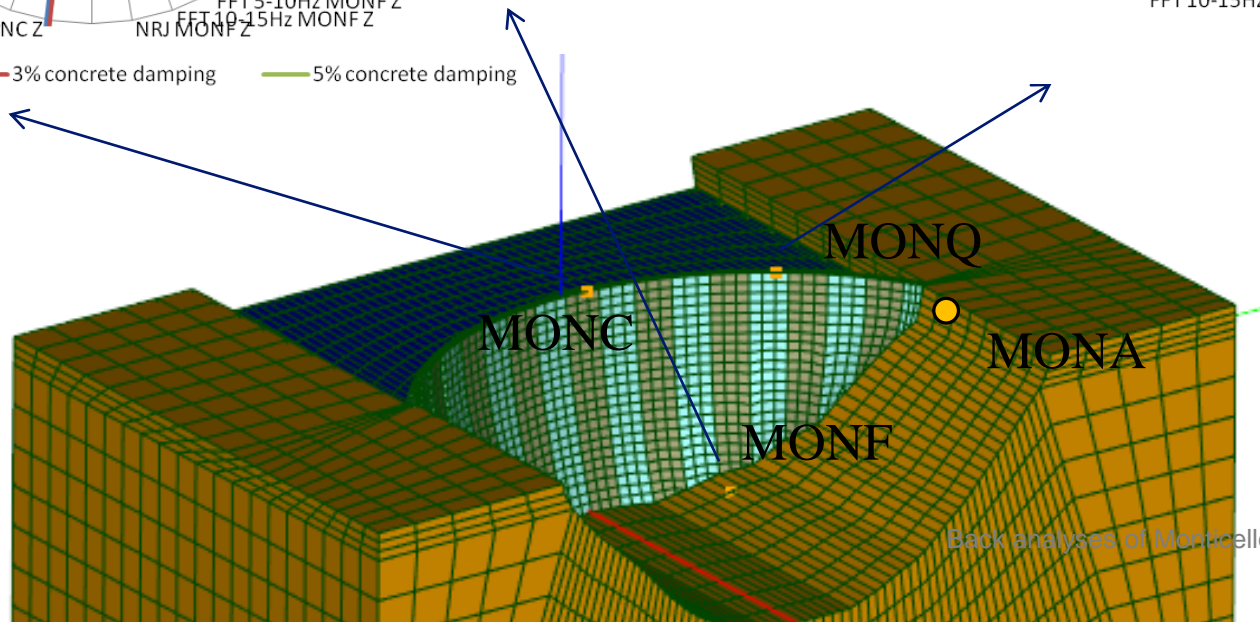
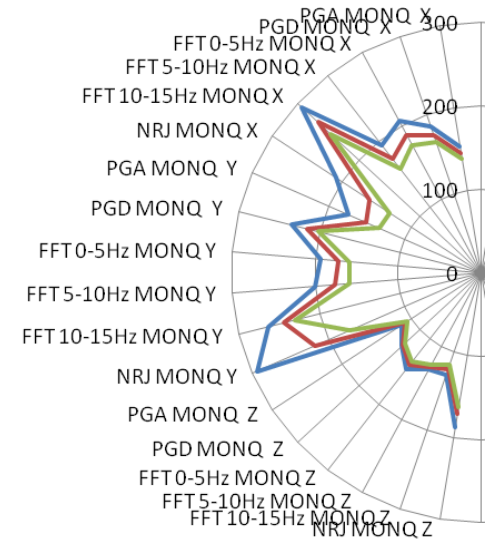
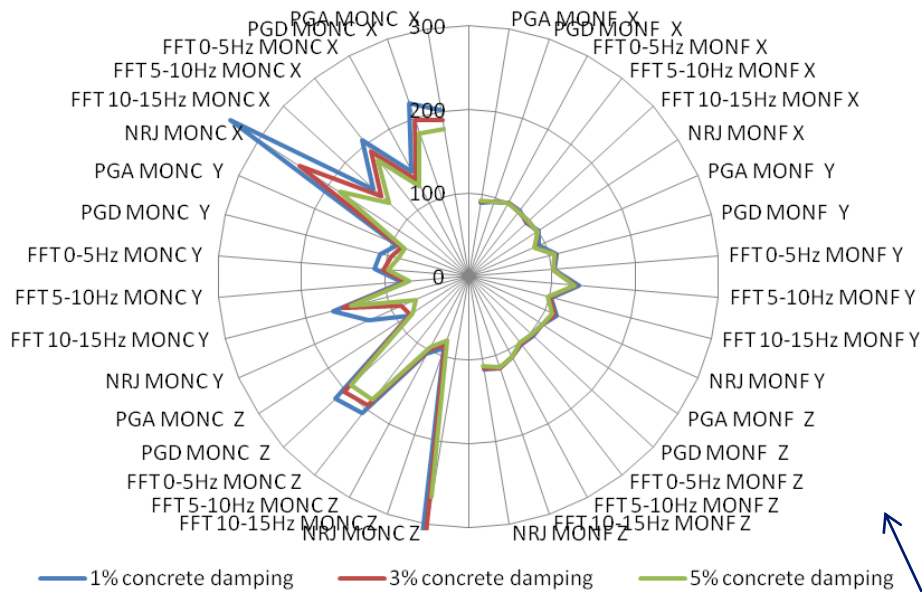
BACK ANALYSIS OF MONTICELLO ARCH DAM

CONCRETE DAMPING : 1%, 3% AND 5%

MONC

MONF

MONQ



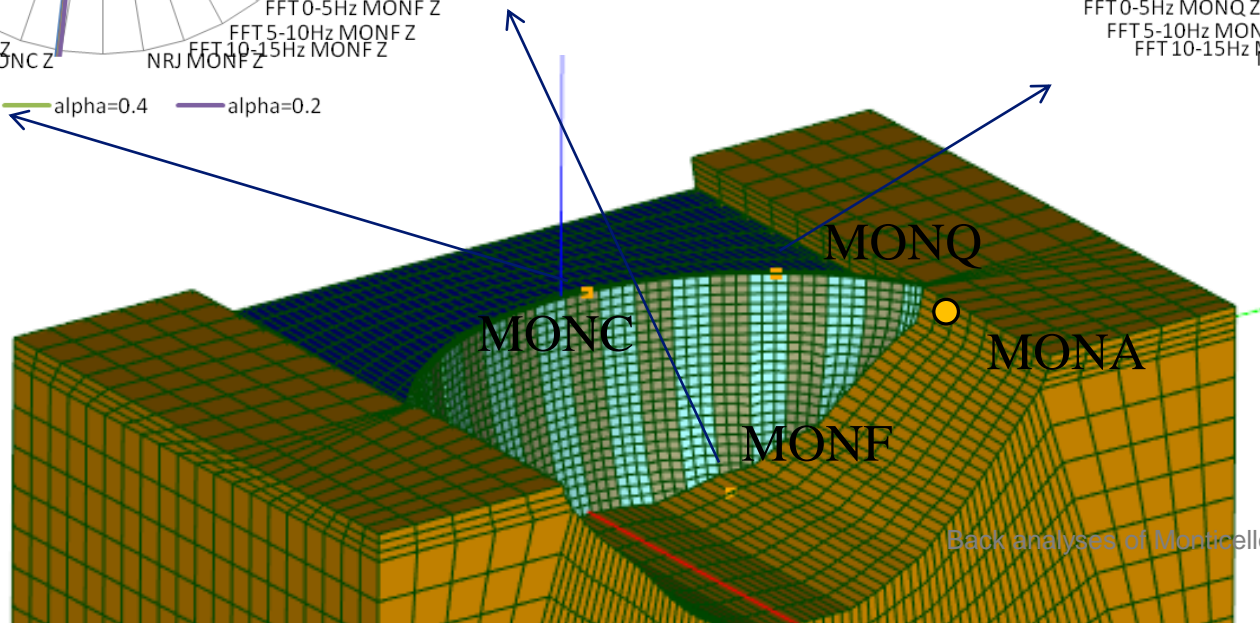
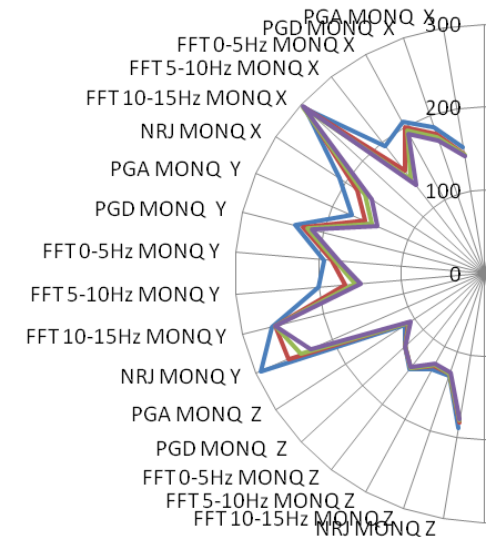
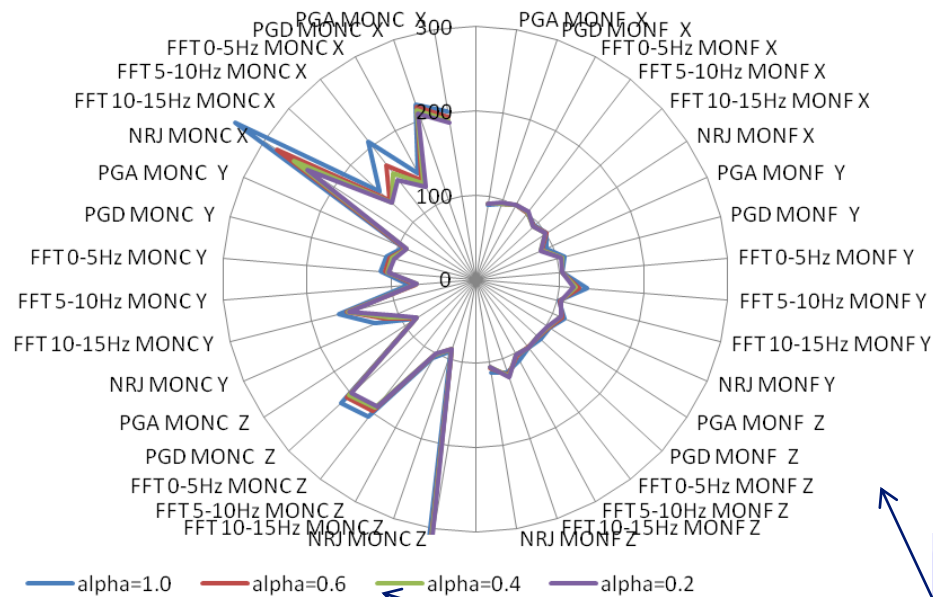
BACK ANALYSIS OF MONTICELLO ARCH DAM

PARTIAL ABSORPTION OF PRESSURE WAVES AT THE RESERVOIR BOTTOM

MONC

MONF

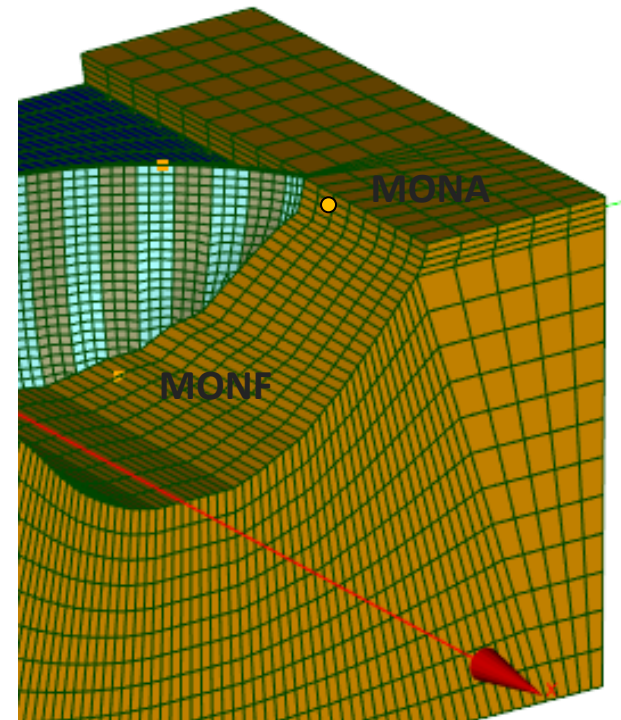
MONQ



BACK ANALYSIS OF MONTICELLO ARCH DAM

EARTHQUAKE INPUT

- At first records at the toe of the dam (MONF) used as input.
- Use of the records on the abutment of the dam (MONA) as input



BACK ANALYSIS OF MONTICELLO ARCH DAM

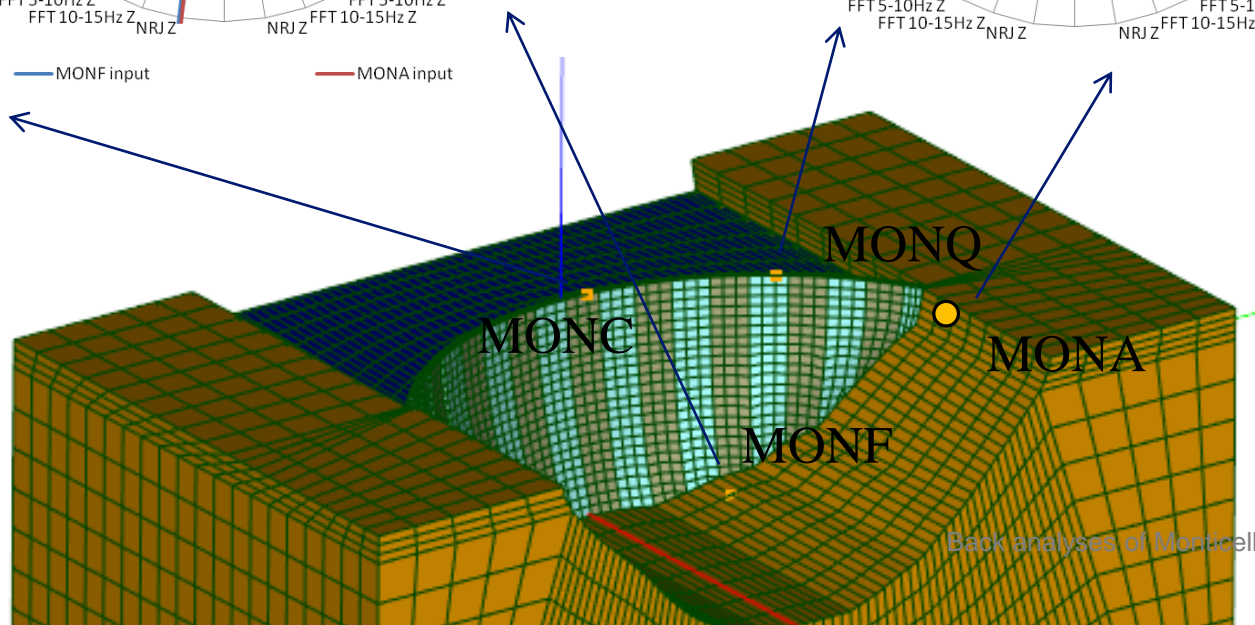
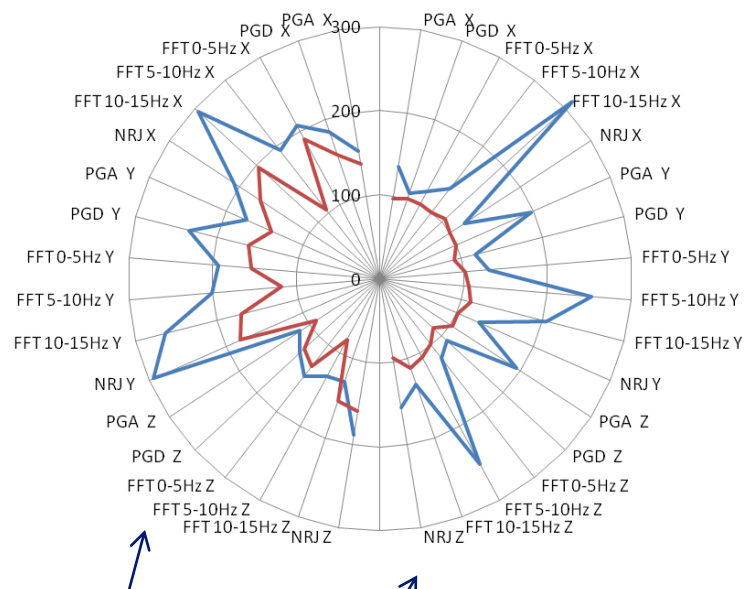
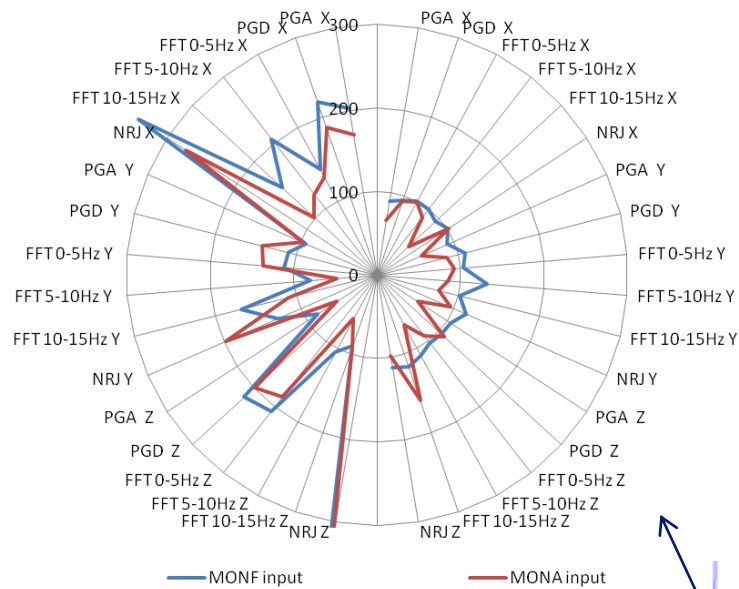
CHANGE OF INPUT

MONC

MONF

MONQ

MONA



BACK ANALYSIS OF MONTICELLO ARCH DAM

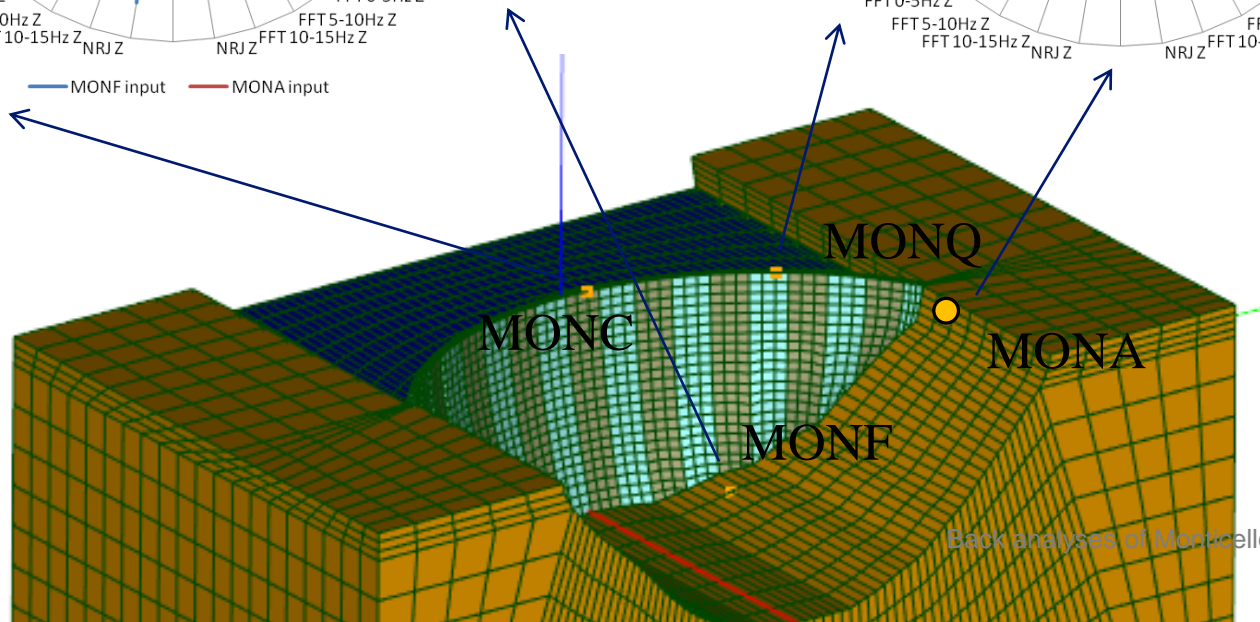
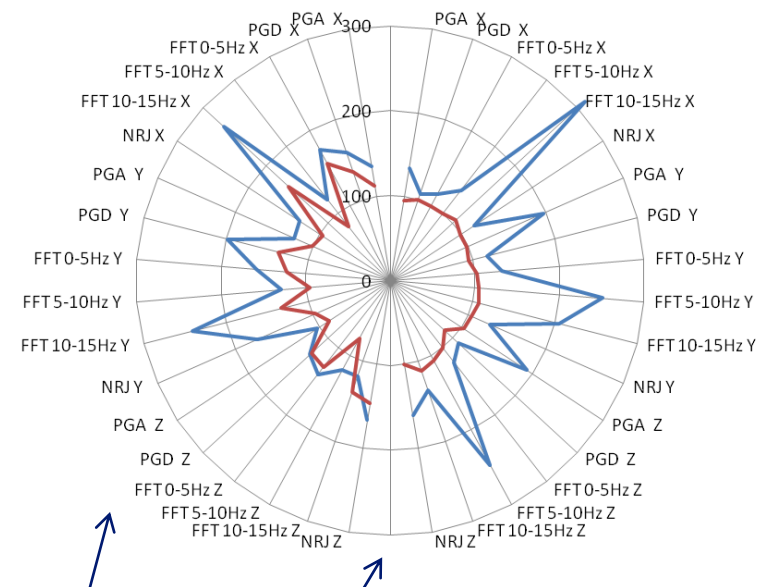
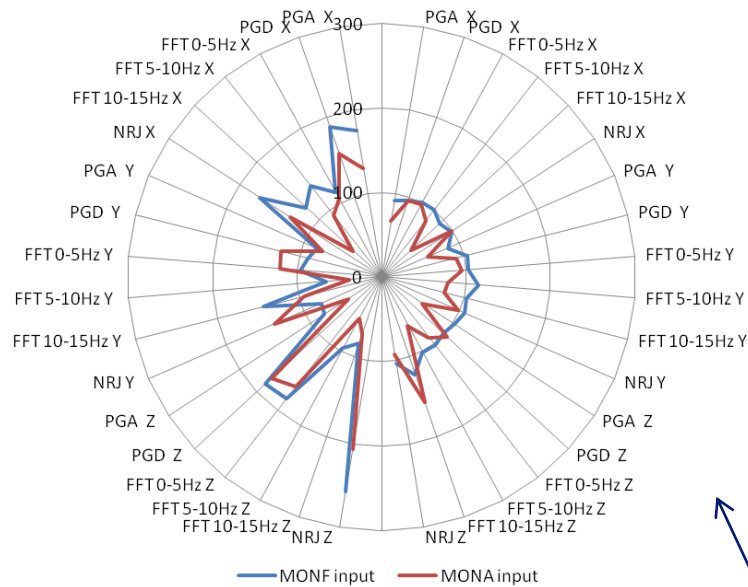
FINAL ASSUMPTIONS : 3% CONCRETE DAMPING / 10000 MPA ROCK / SEDIMENT ABSORPTION

MONC

MONF

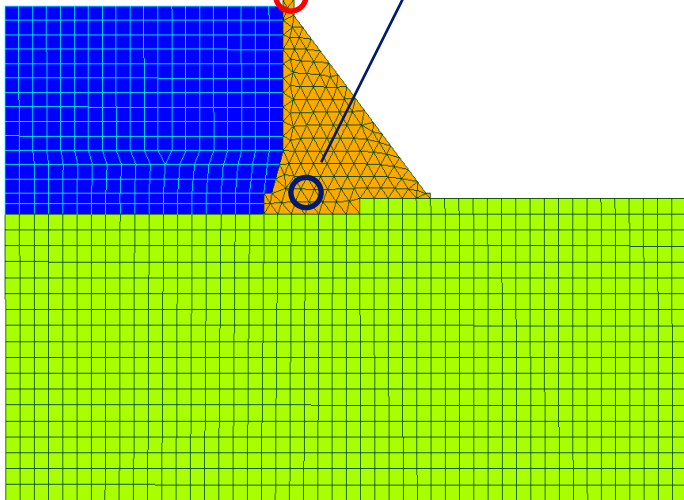
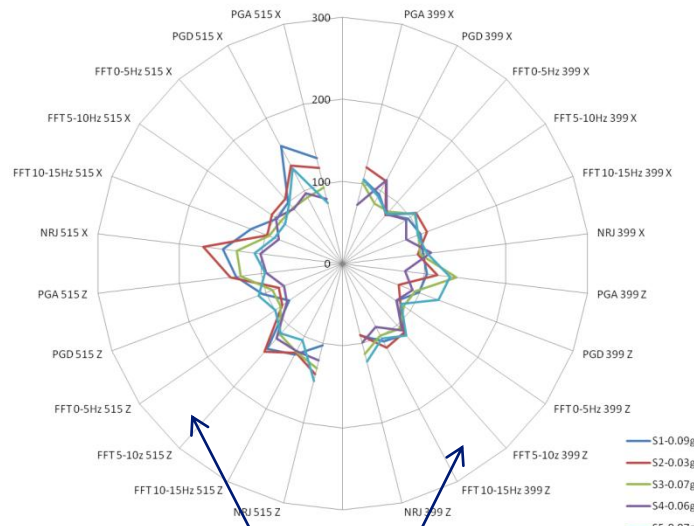
MONQ

MONA

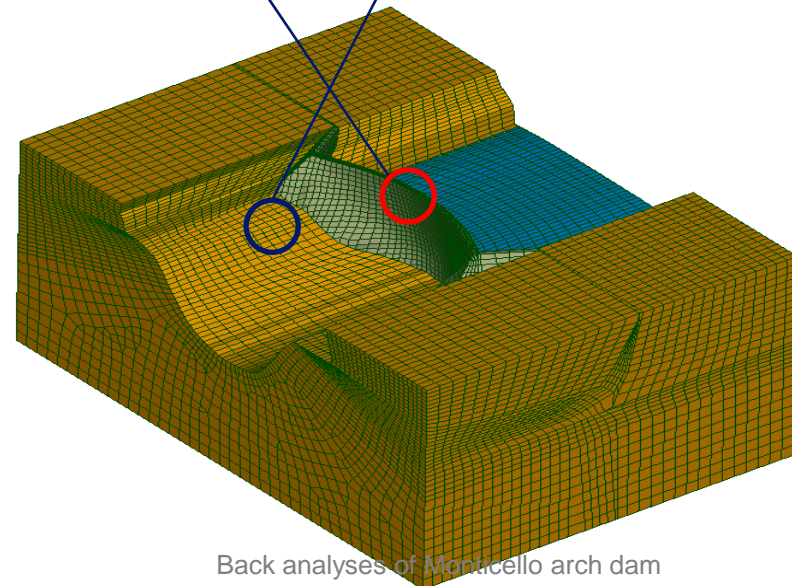
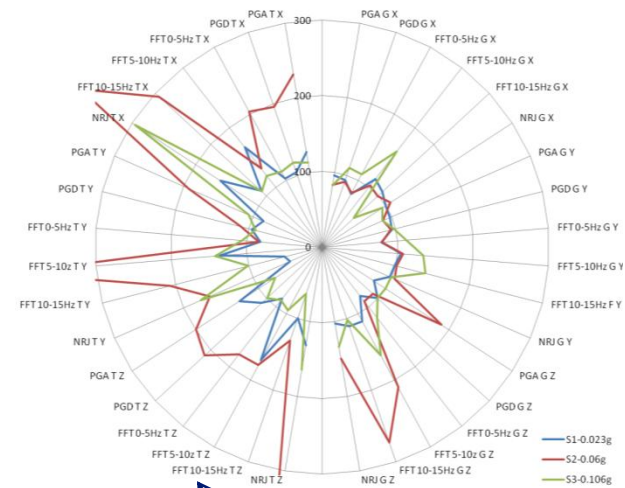


BACK ANALYSIS OF OTHER DAMS

TAGOKURA GRAVITY DAM AND KUROBE ARCH DAM FOR SEVERAL EARTHQUAKES



Overall damping between 10-20 % !)



Back analyses of Monticello arch dam

CONCLUSION

- **Development of FE approach to take into account soil-structure and fluid-structure interaction**
 - Based on verified bibliography
 - Validated on test case
 - Available in the EDF finite-element software Code_Aster (for engineers)
- **Comparison with records on Monticello dam :**
 - Blind prediction roughly overestimates 2x the response of the dam
 - Additional improvements but still overestimates by factor 1.5
 - Small earthquake : but let's try to be good with small before big
- **More details about similar studies**
 - 16th World Conference of Earthquake engineering in Chile
 - Next 2017 USSD conference

CONCLUSION

- **More comparisons between FE analyses and records on dams are needed but this requires :**
 - Complex FE analyses
 - Seismic data processing
 - Concrete dam's behavior knowledge
 - And....

EARTHQUAKES RECORDS ON DAMS !!!

- Records can really help engineers to built better model to assess the safety of dams under earthquake (and avoid overestimation of the dam's response)
- Can be temporary
- **Concrete dams behave well under earthquake : let's prove it !**



Merci de votre attention